

***GEOSCIENCE GROUP, INC.***  
**INVESTIGATIVE PROCEDURES**  
Building 42 Additions & Renovations  
Geoscience Project No. CH09.0088.GE

Page 1 Of 2

**FIELD**

**Soil Test Borings:** Eight (8) soil test borings (B-1 through B-8) were drilled at the approximate locations shown on the attached Test Location Diagram, Drawing No. CH09.0088.GE-1. Soil sampling and penetration testing were performed in accordance with ASTM D 1586-84.

The borings were advanced with hollow-stem, continuous-flight augers and, at standard intervals, soil samples were obtained with a standard 1.4-inch (3.6cm) I.D., 2-inch (5.1cm) O.D., split-tube sampler. The sampler was first seated 6 inches (15.2cm) to penetrate any loose cuttings, then driven an additional 12 inches (30.5cm) with blows of a 140 pound (63.5kg) hammer falling 30 inches (76.2cm). The number of hammer blows required to drive the sampler the final 12 inches (30.5cm) was recorded and is designated the "Standard Penetration Resistance" (N-Value). The Standard Penetration Resistance, when properly evaluated, is an index to soil strength, density and ability to support foundations.

Representative portions of each soil sample were placed in glass jars and taken to our laboratory. The samples were then examined by an engineer to verify the driller's field classifications. Test Boring Records are attached indicating the soil descriptions and Standard Penetration Resistances.

**LABORATORY**

**Moisture Content:** The moisture content is the ratio, expressed as a percentage, of the weight of the water in a given mass of soil to the weight of the solid particles. These tests were conducted in accordance with ASTM Designation D 2216-66. The test results are presented on the attached sheets.

**Soil Plasticity Test (Atterberg Limits Test):** A representative sample of the onsite soils was selected for Atterberg Limits testing to determine the soil's plasticity characteristics. The Plasticity Index (PI) is representative of this characteristic and is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL). The Liquid Limit is the moisture content at which the soil will flow as a heavy viscous fluid and is determined in accordance with ASTM D 423. The Plastic Limit is the moisture content at which the soil begins to lose its plasticity and is determined in accordance with ASTM D 424. The data obtained is presented on the attached sheets.

**Compaction Test:** A representative sample of the onsite soils was obtained from the soil test borings to determine their suitability as fill material. A Standard Proctor Compaction Test (ASTM D 698) was performed on these soils to determine its compaction characteristics, including maximum dry density and optimum moisture content. The test results are presented on the attached sheets.

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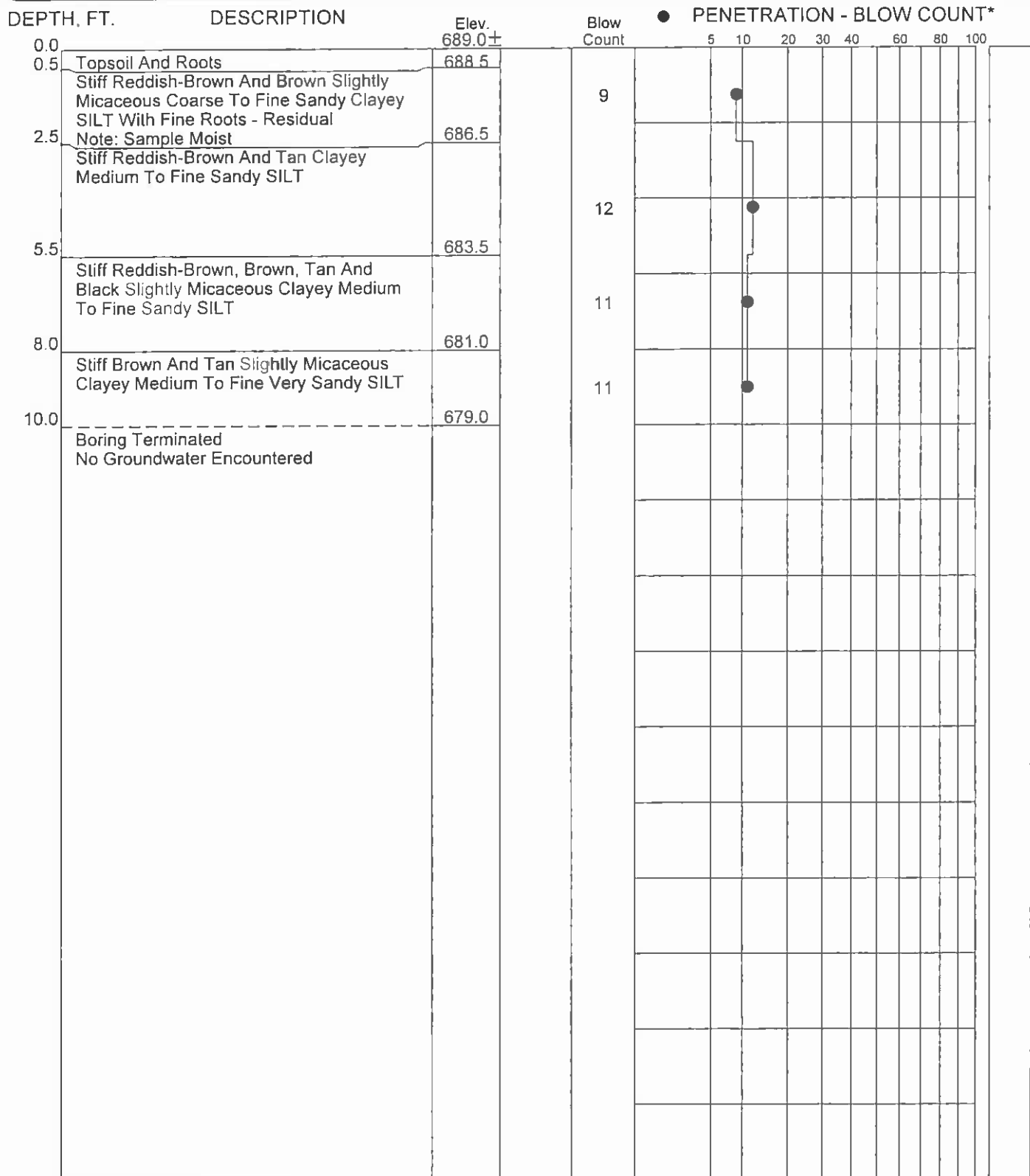
Page 2 Of 2

California Bearing Ratio (CBR): The results of the compaction test described above were utilized in compacting samples for laboratory CBR tests. The California Bearing Ratio is a punching shear test which provides a semi-empirical index of the strength and deflection characteristics of a soil which has been correlated with pavement performance. The test is performed on a six (6) inch diameter, 4.61 inch thick disc of compacted soil that is confined in a steel cylinder. Before testing, the sample is inundated under a confining pressure approximately equal to the weight of the future pavement in order to determine the potential swelling, and to simulate the worst case conditions that can occur in the field. A piston approximately two (2) inches in diameter is then forced into the soil at a standard rate to determine the resistance to penetration. The CBR value is the ratio expressed as a percentage of the actual load required to produce a 0.1 inch deflection to that required to produce the same deflection in a standardized crushed stone. The results of the CBR tests are shown on the attached sheets.

BORING NO.: **B-1**  
 DATE DRILLED: **11/13/09**  
 DRILLING CONTRACTOR: **Soil Drilling Services**  
 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**







**TEST  
BORING  
RECORD**

**GEOSCIENCE  
GROUP INC.**



BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

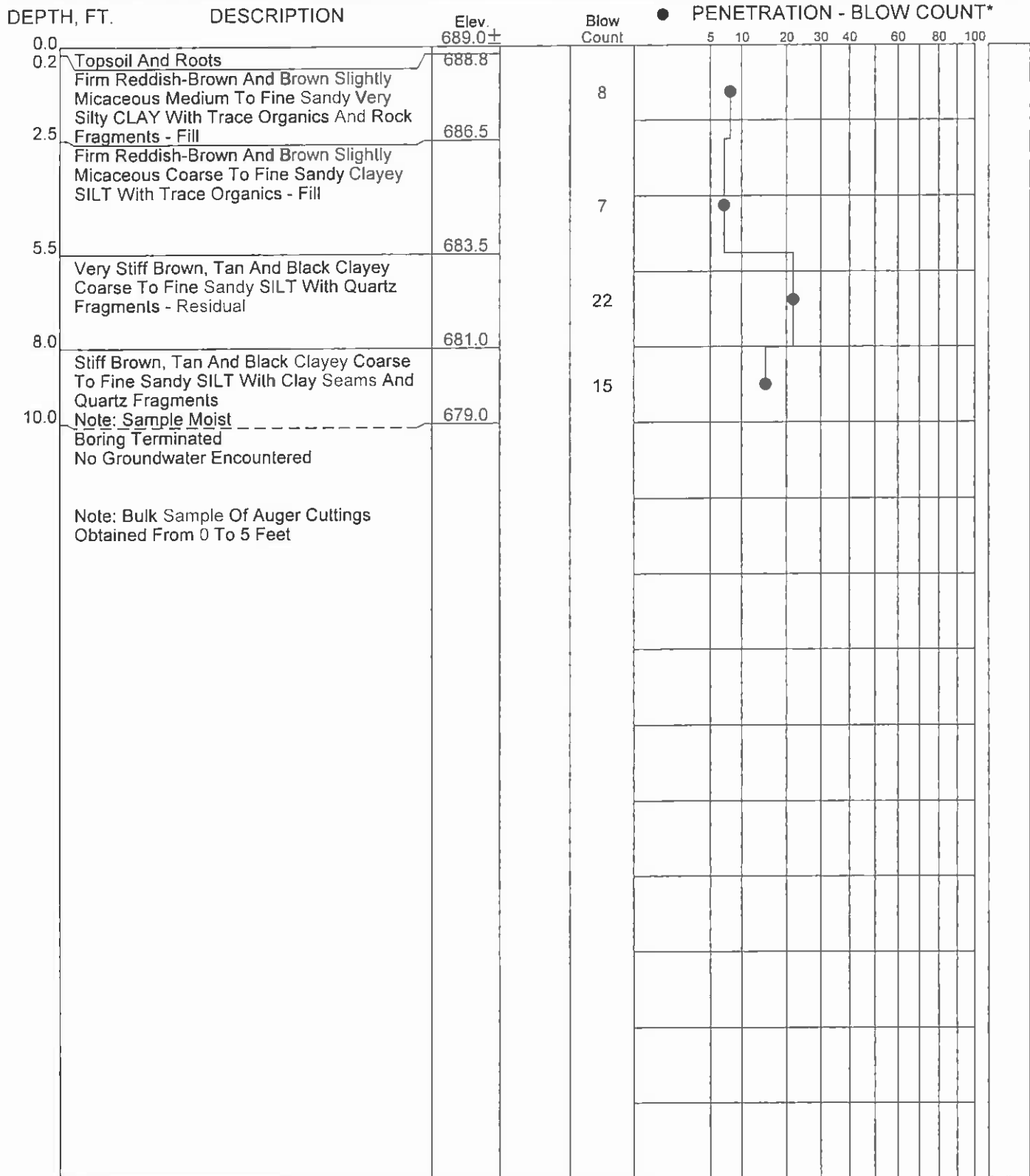
\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg) HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN. (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

 PRESSUREMETER TEST    
  WATER TABLE - 24 HR.  
 ROCK CORE RECOVERY    
  WATER TABLE - 1 HR.  
 LOSS OF DRILLING WATER    
  CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER    
 PAGE **1 of 1**

BORING NO.: **B-2**  
 DATE DRILLED: **11/13/09**  
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 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**

# TEST BORING RECORD

**GEOSCIENCE  
GROUP INC.**



BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

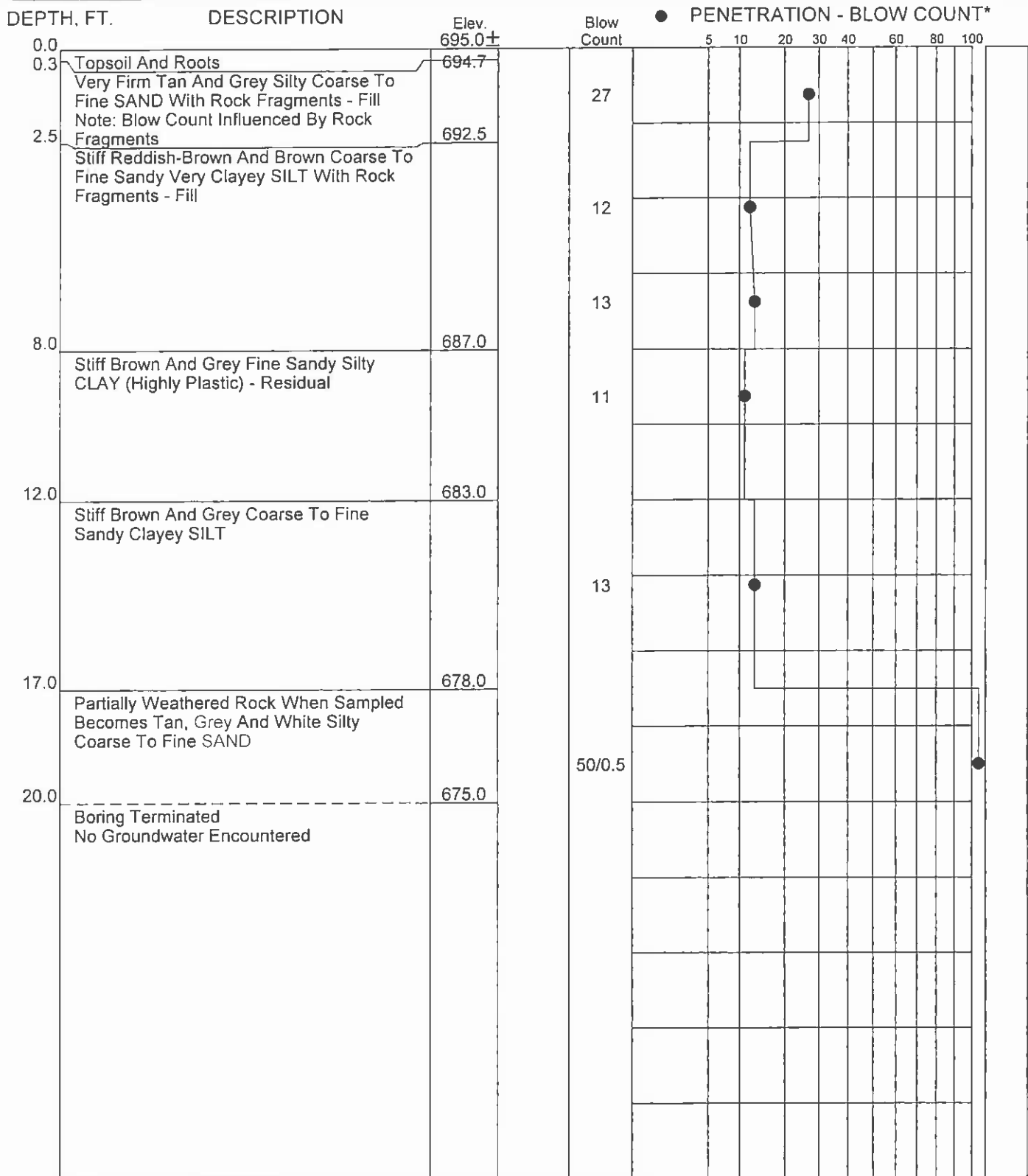
\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg) HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN. (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

PRESSUREMETER TEST  
 ROCK CORE RECOVERY  
 LOSS OF DRILLING WATER  
 WATER TABLE - 24 HR.  
 WATER TABLE - 1 HR.  
 CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER

BORING NO.: **B-3**  
 DATE DRILLED: **11/13/09**  
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**TEST  
BORING  
RECORD**

**GEOSCIENCE  
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BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

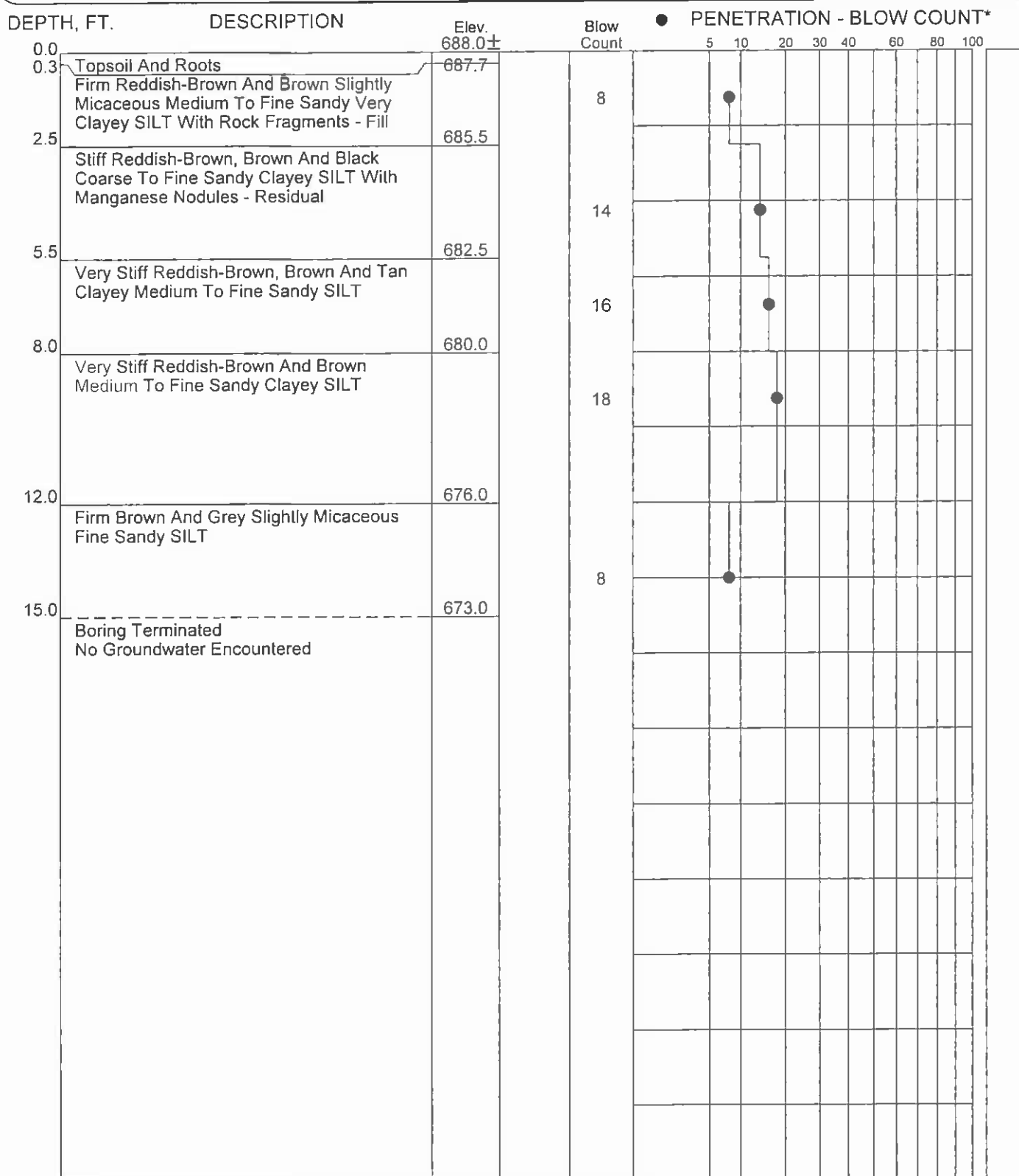
\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg) HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN. (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

■ PRESSUREMETER TEST      ≡ WATER TABLE - 24 HR.  
 |50%| ROCK CORE RECOVERY      ≡ WATER TABLE - 1 HR.  
 ◀ LOSS OF DRILLING WATER      ■ CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER      PAGE 1 of 1

BORING NO.: **B-4**  
 DATE DRILLED: **11/13/09**  
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 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**

# TEST BORING RECORD

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







BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg)  
 HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN.  
 (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

PRESSUREMETER TEST  
 ROCK CORE RECOVERY  
 LOSS OF DRILLING WATER  
 WATER TABLE - 24 HR.  
 WATER TABLE - 1 HR.  
 CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER

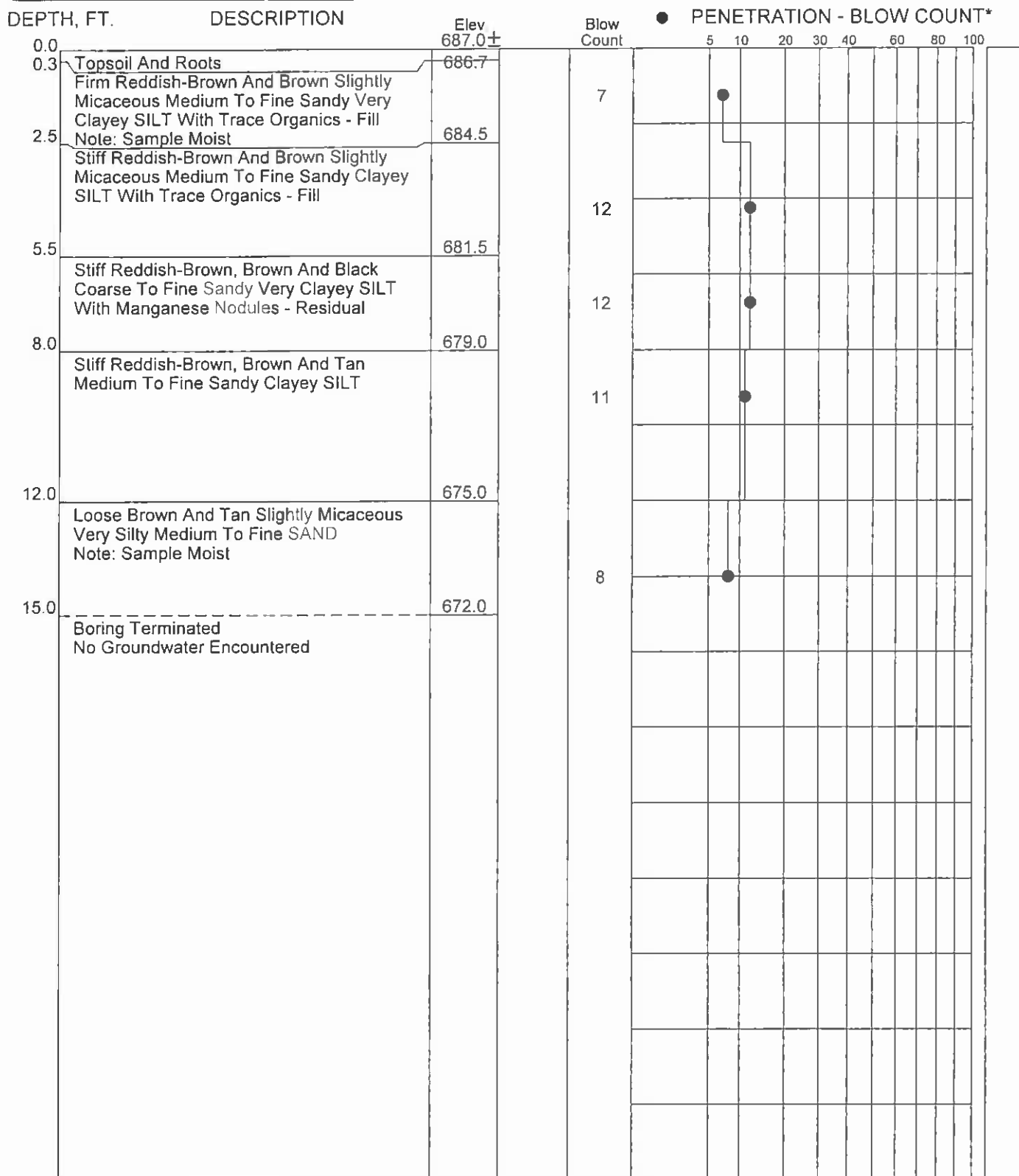
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	PRESSUREMETER TEST		WATER TABLE - 24 HR.
	ROCK CORE RECOVERY		WATER TABLE - 1 HR.
	LOSS OF DRILLING WATER		CAVE-IN DEPTH
WOH	WEIGHT OF HAMMER	PAGE	1 of 1

BORING NO.: **B-6**  
 DATE DRILLED: **11/13/09**  
 DRILLING CONTRACTOR: **Soil Drilling Services**  
 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**

**TEST  
BORING  
RECORD**

**GEOSCIENCE  
GROUP INC.**



BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg)  
 HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN.  
 (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)



PRESSUREMETER TEST



ROCK CORE RECOVERY



LOSS OF DRILLING WATER

WOH

WEIGHT OF HAMMER



WATER TABLE - 24 HR.



WATER TABLE - 1 HR.



CAVE-IN DEPTH

PAGE

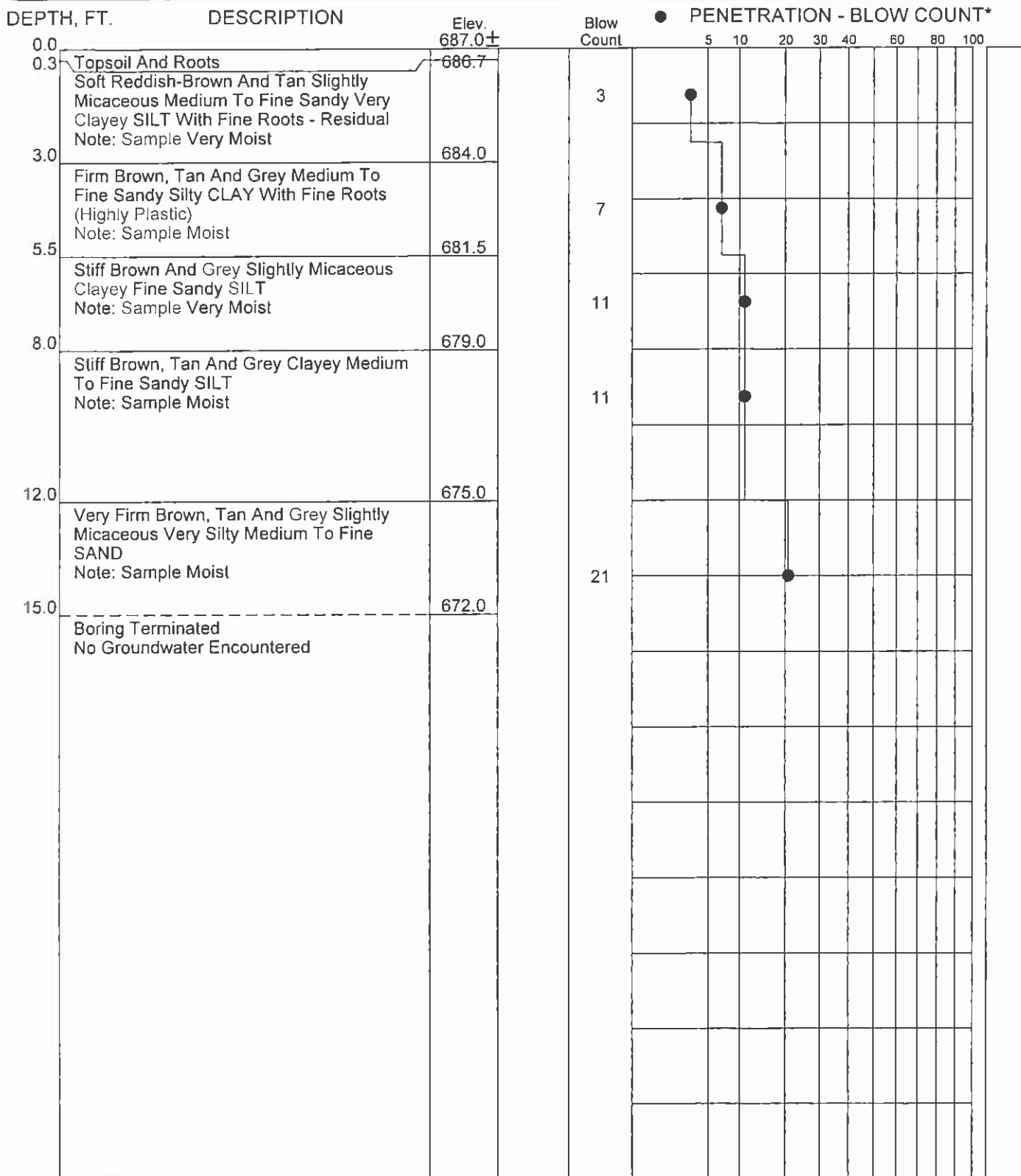
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BORING NO.: **B-7**  
 DATE DRILLED: **11/13/09**  
 DRILLING CONTRACTOR: **Soil Drilling Services**  
 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**

# TEST BORING RECORD

**GEOSCIENCE  
GROUP INC.**



BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

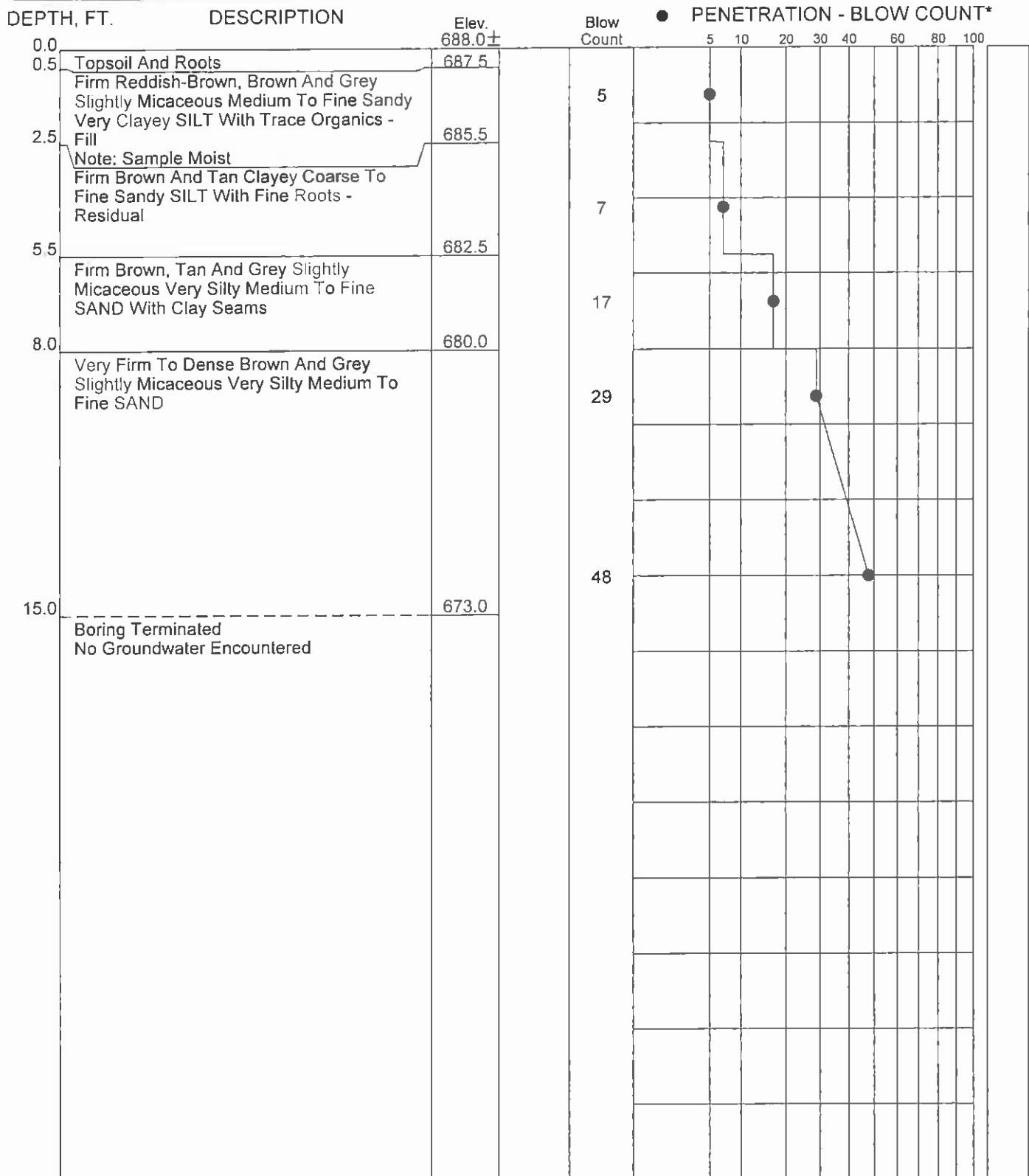
\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg) HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN. (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

PRESSUREMETER TEST  
 ROCK CORE RECOVERY  
 LOSS OF DRILLING WATER  
 WATER TABLE - 24 HR.  
 WATER TABLE - 1 HR.  
 CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER

BORING NO.: **B-8**  
 DATE DRILLED: **11/13/09**  
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 JOB NO.: **CH09.0088.GE**  
 PROJECT: **BUILDING 42 ADDITIONS & RENOVATIONS**

# TEST BORING RECORD

**GEOSCIENCE  
GROUP INC.**



BORING AND SAMPLING MEETS ASTM D-1586  
 CORE DRILLING MEETS ASTM D-2113

\*PENETRATION IS THE NUMBER OF BLOWS OF A 140 LB. (63.5kg) HAMMER FALLING 30 IN. (76.2cm) REQUIRED TO DRIVE A 1.4 IN. (3.6cm) I.D. SAMPLER 1 FT. (30.5cm)

■ PRESSUREMETER TEST    ≡ WATER TABLE - 24 HR.  
 |50% ROCK CORE RECOVERY    ≡ WATER TABLE - 1 HR.  
 ◀ LOSS OF DRILLING WATER    ■ CAVE-IN DEPTH  
 WOH WEIGHT OF HAMMER    PAGE **1 of 1**

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Charlotte, North Carolina

**COMPACTION  
TEST**

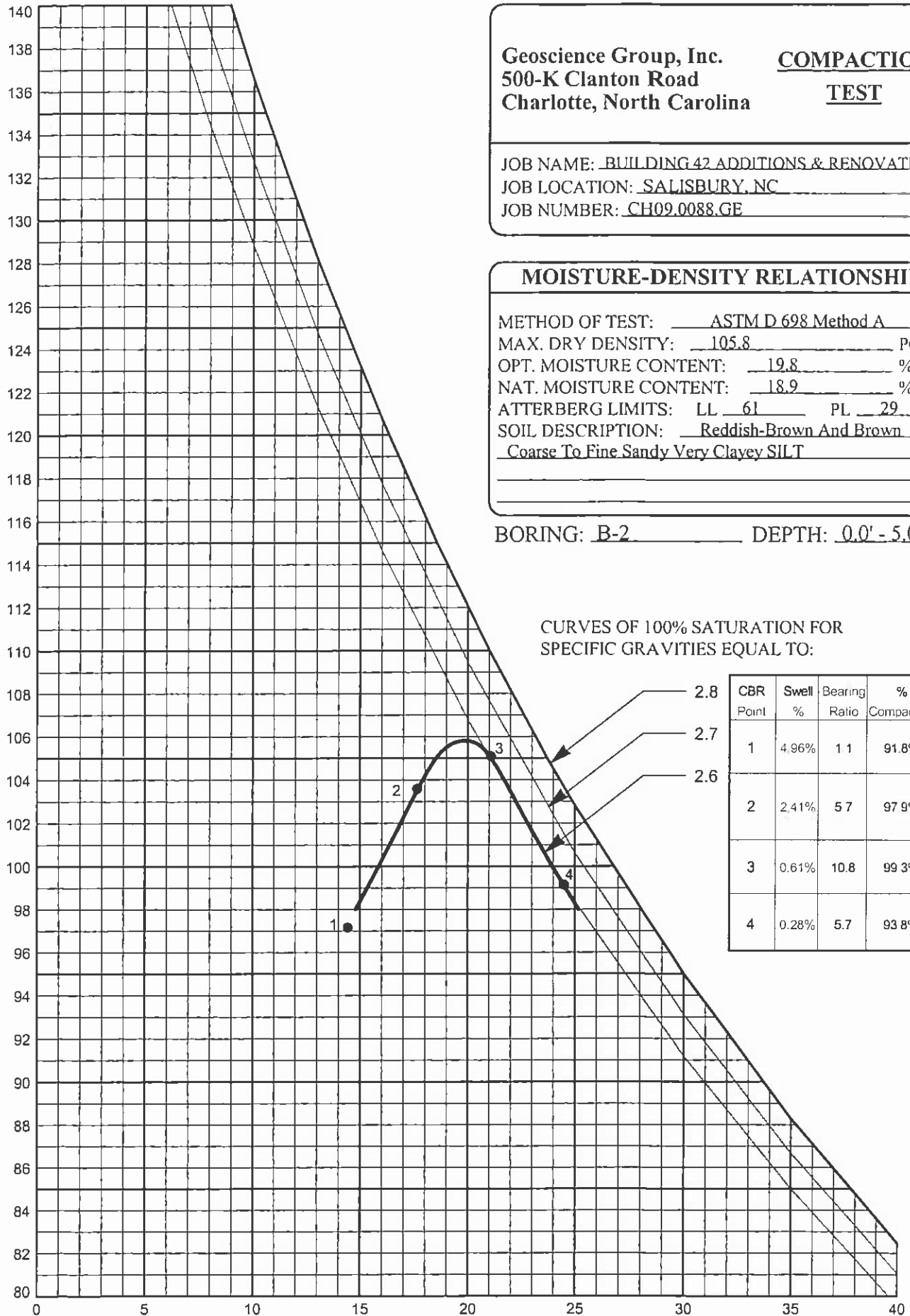
JOB NAME: BUILDING 42 ADDITIONS & RENOVATIONS  
JOB LOCATION: SALISBURY, NC  
JOB NUMBER: CH09.0088.GE

**MOISTURE-DENSITY RELATIONSHIP**

METHOD OF TEST: ASTM D 698 Method A  
MAX. DRY DENSITY: 105.8 PCF  
OPT. MOISTURE CONTENT: 19.8 %  
NAT. MOISTURE CONTENT: 18.9 %  
ATTERBERG LIMITS: LL 61 PL 29  
SOIL DESCRIPTION: Reddish-Brown And Brown  
Coarse To Fine Sandy Very Clayey SILT

BORING: B-2 DEPTH: 0.0' - 5.0'

DRY UNIT WEIGHT (pounds per cubic foot)



CURVES OF 100% SATURATION FOR  
SPECIFIC GRAVITIES EQUAL TO:

CBR Point	Swell %	Bearing Ratio	% Compaction
1	4.96%	1.1	91.8%
2	2.41%	5.7	97.9%
3	0.61%	10.8	99.3%
4	0.28%	5.7	93.8%

WATER CONTENT (percent of dry weight)

## CALIFORNIA BEARING RATIO TEST

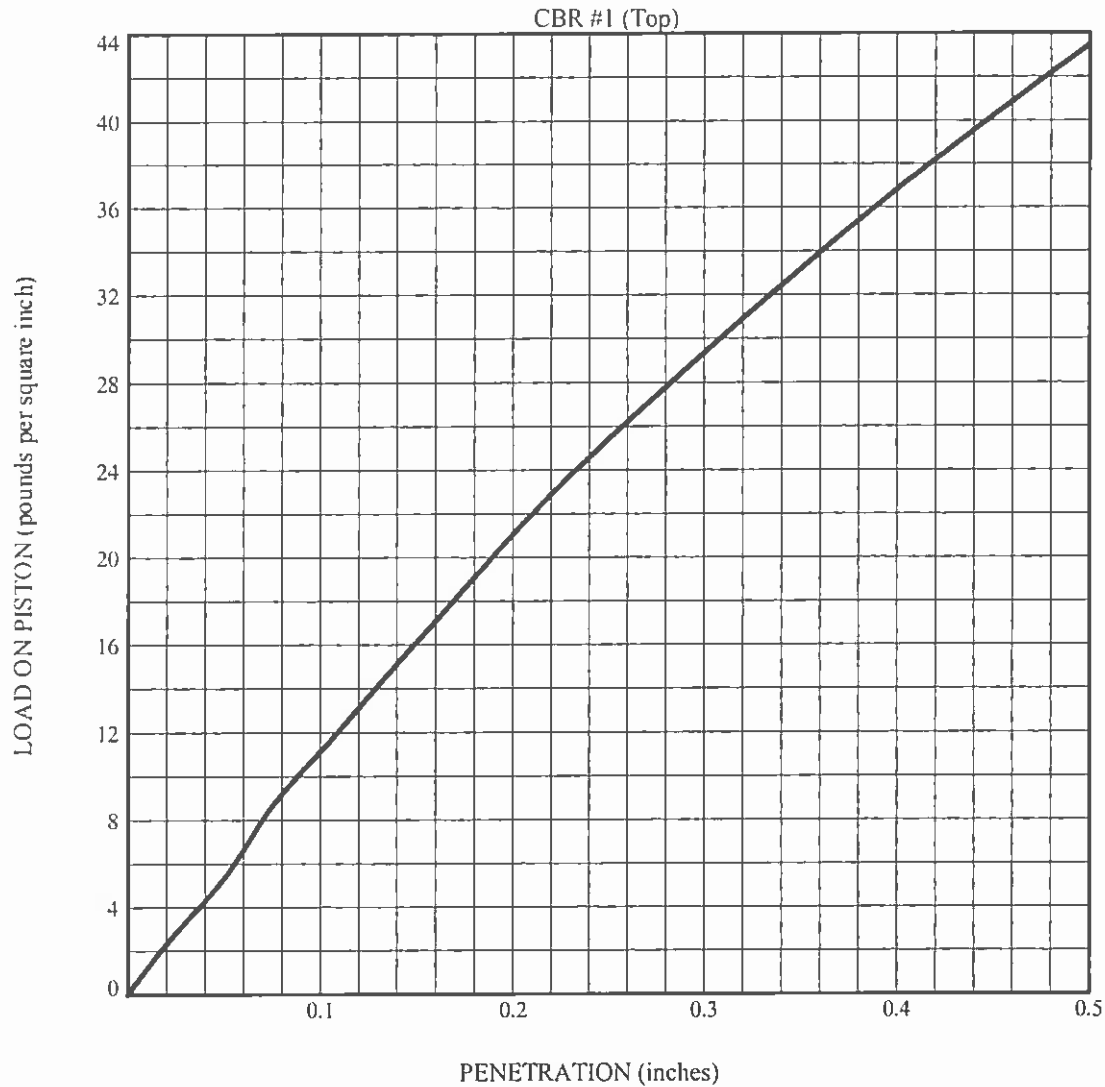
JOB NAME: BUILDING 42 ADDITIONS & RENOVATIONS

JOB NUMBER: CH09.0088.GE

JOB LOCATION: SALISBURY, NC

BORING NUMBER: B-2

BORING DEPTH: 0.0' - 5.0'



1. Method of Preparation ASTM D 698 Method C
2. Description of Sample Reddish-Brown And Brown Coarse To Fine Sandy  
Very Clayey SILT
3. Dry Density Before Soaking 97.1 PCF
4. Swell 4.96%
5. Percent of Optimum Standard Compaction 91.8%
6. Moisture Content 14.4%
7. Bearing Ratio @ 0.1 Inch 1.1
8. Surcharge 72.6 PSF

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**Charlotte, North Carolina**

## CALIFORNIA BEARING RATIO TEST

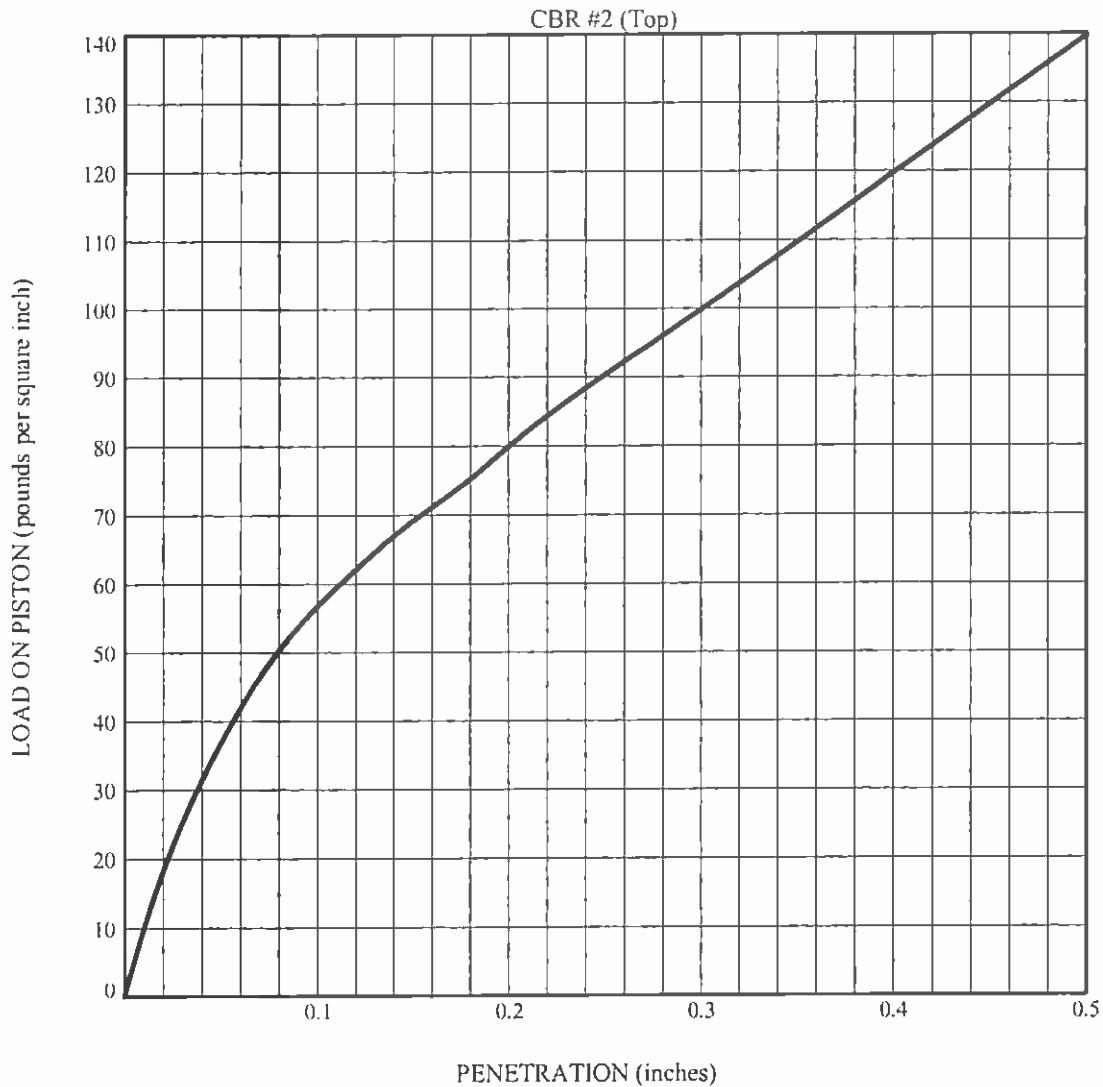
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JOB NUMBER: CH09.0088.GE

JOB LOCATION: SALISBURY, NC

BORING NUMBER: B-2

BORING DEPTH: 0.0' - 5.0'



- |   |  |
|---|--|
| 1. Method of Preparation                  | <u>ASTM D 698 Method C</u>   |
| 2. Description of Sample                  | <u>Reddish-Brown And Brown Coarse To Fine Sandy</u><br><u>Very Clayey SILT</u> |
| 3. Dry Density Before Soaking             | <u>103.6 PCF</u>   |
| 4. Swell                                  | <u>2.41%</u>   |
| 5. Percent of Optimum Standard Compaction | <u>97.9%</u>   |
| 6. Moisture Content                       | <u>17.7%</u>   |
| 7. Bearing Ratio @ 0.1 Inch               | <u>5.7</u>   |
| 8. Surcharge                              | <u>72.6 PSF</u>  |

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**Charlotte, North Carolina**

## CALIFORNIA BEARING RATIO TEST

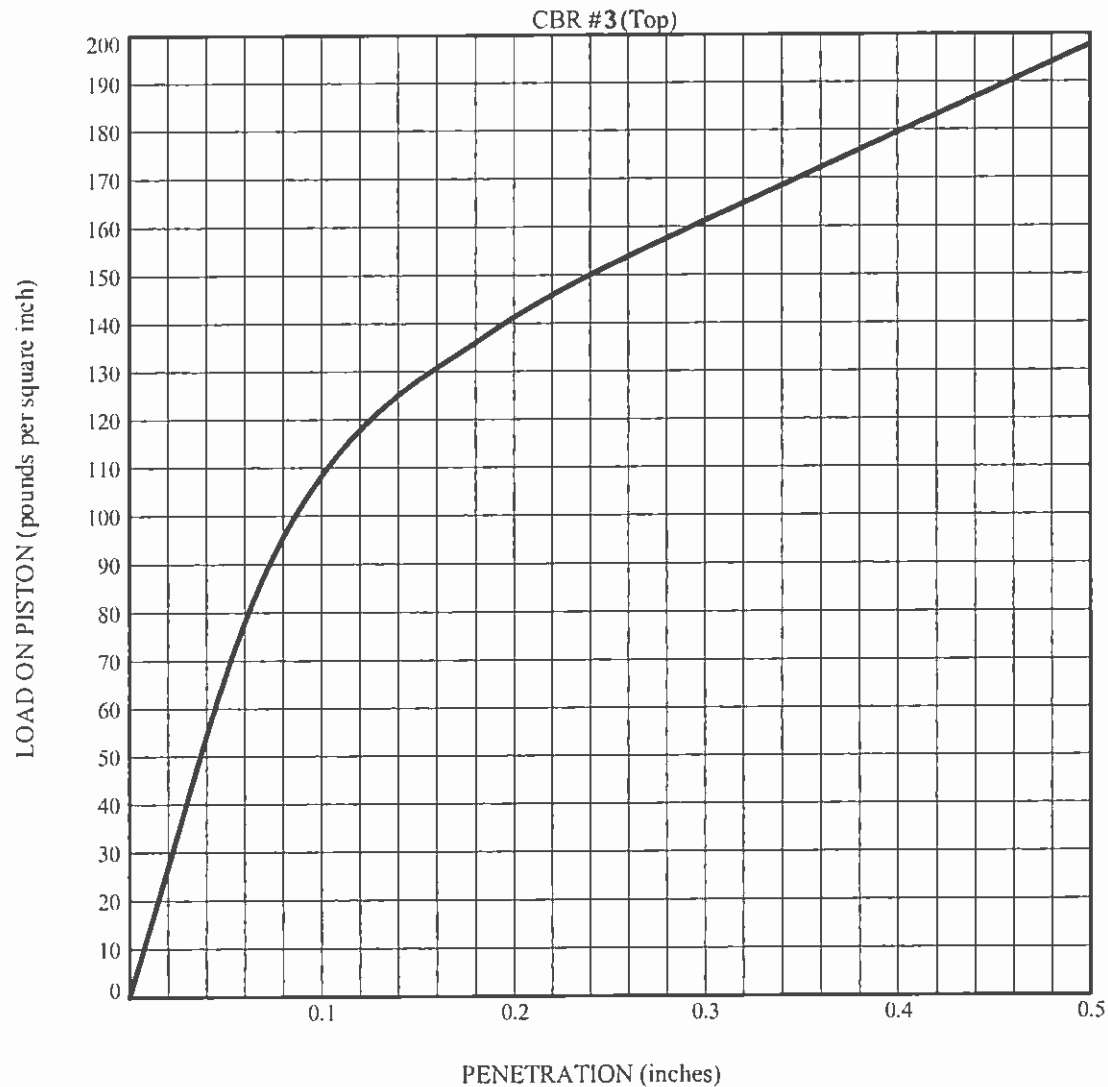
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JOB NUMBER: CH09.0088.GE

JOB LOCATION: SALISBURY, NC

BORING NUMBER: B-2

BORING DEPTH: 0.0' - 5.0'



1. Method of Preparation ASTM D 698 Method C
2. Description of Sample Reddish-Brown And Brown Coarse To Fine Sandy  
Very Clayey SILT
3. Dry Density Before Soaking 105.1 PCF
4. Swell 0.61%
5. Percent of Optimum Standard Compaction 99.3%
6. Moisture Content 21.1%
7. Bearing Ratio @ 0.1 Inch 10.8
8. Surcharge 72.6 PSF

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## CALIFORNIA BEARING RATIO TEST

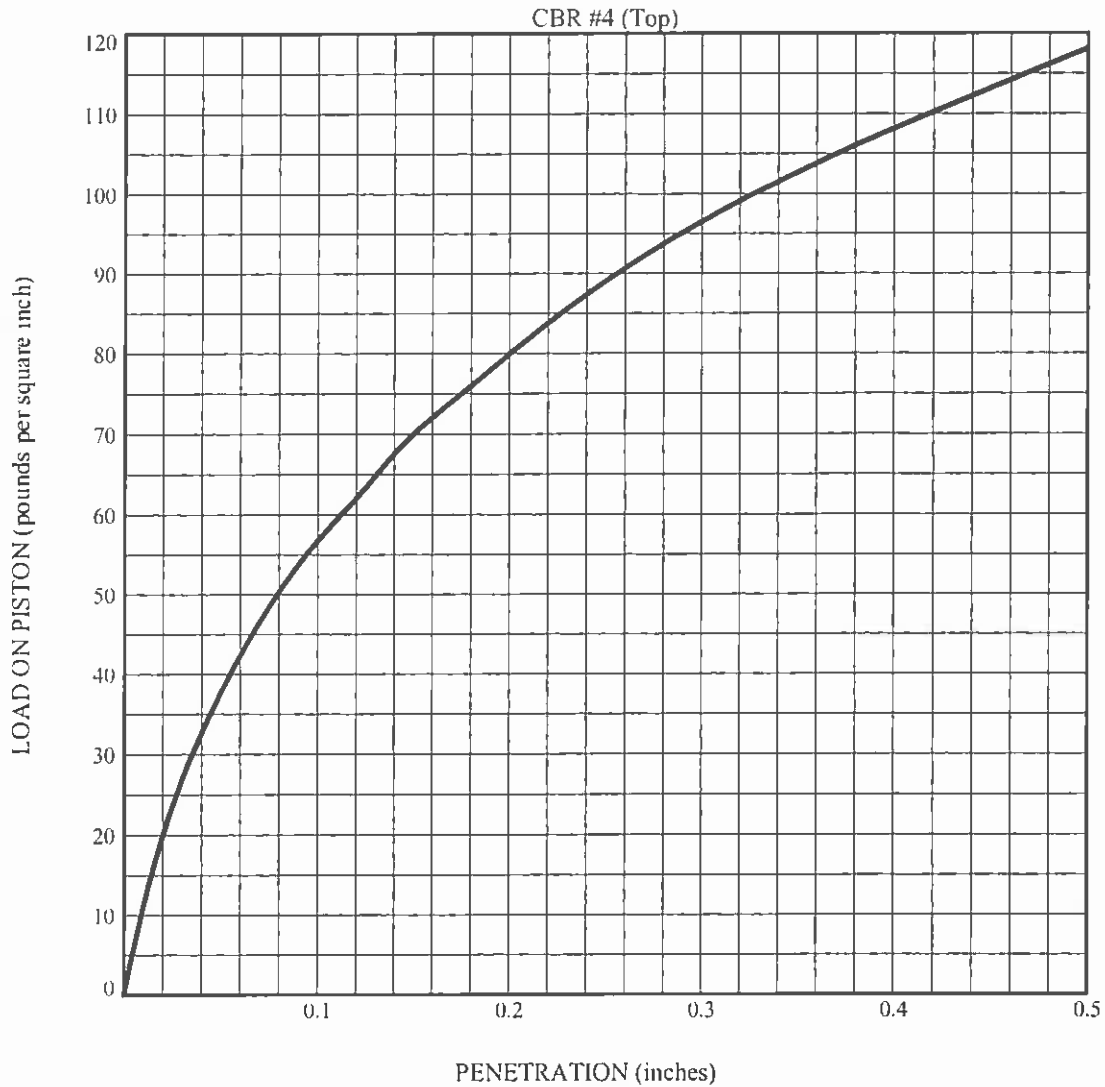
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JOB NUMBER: CH09.0088.GE

JOB LOCATION: SALISBURY, NC

BORING NUMBER: B-2

BORING DEPTH: 0.0' - 5.0'



1. Method of Preparation ASTM D.698 Method C
2. Description of Sample Reddish-Brown And Brown Coarse To Fine Sandy  
Very Clayey SILT
3. Dry Density Before Soaking 99.2 PCF
4. Swell 0.28%
5. Percent of Optimum Standard Compaction 93.8%
6. Moisture Content 24.5%
7. Bearing Ratio @ 0.1 Inch 5.7
8. Surcharge 72.6 PSF

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